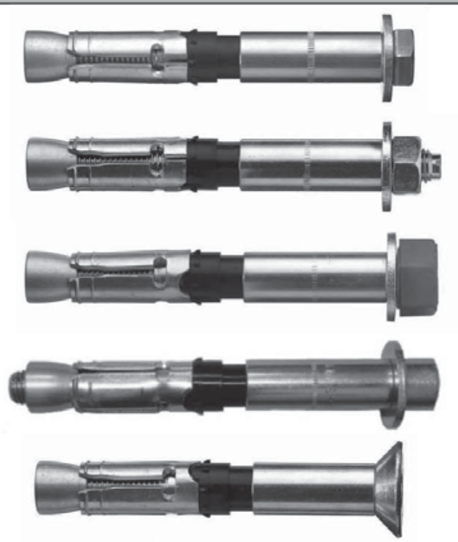
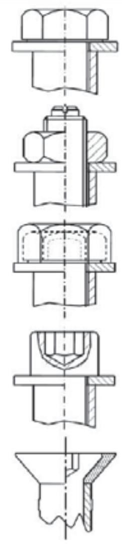
HSL-3 Heavy duty anchor, carbon steel

Anchor version	Benefits
  HSL-3 Bolt version HSL-3-G Threaded rod version HSL-3-B Safety cap version HSL-3-SH Hexagonal socked head screws HSL-3-SK Countersunk version	- suitable for non-cracked and cracked concrete C 20/25 to C 50/60 - high loading capacity - force-controlled expansion - reliable pull-down of the part fastened - no rotation in hole when tightening bolt

Concrete | Tensile zone | Seismic | Fatigue | Shock | Fire resistance | European Technical Approval | CE conformity | PROFIS Anchor design software

Approvals / certificates

Description	Authority / Laboratory	No. / date of issue
European technical approval [a)]	CSTB, Paris	ETA-02/0042 / 2013-01-10
ICC-ES report incl. seismic	ICC evaluation service	ESR 1545 / 2014-02-01
Shockproof fastenings in civil defence installations	Federal Office for Cicil Protection, Bern	BZS D 08-601 / 2008-06-30
Fire test report	IBMB, Braunschweig	UB 3041/1663-CM / 2004-03-22
Assessment report (fire)	warringtonfire	WF 327804/A / 2013-07-10

a) All data given in this section according ETA-02/0042, issue 2013-01-10.

Basic loading data (for a single anchor)

All data in this section applies to

For details see Simplified design method

- Correct setting (See setting instruction)
- No edge distance and spacing influence
- Concrete as specified in the table
- *Steel* failure
- Minimum base material thickness
- Concrete C 20/25, $f_{ck,cube}$ = 25 N/mm²

Mean ultimate resistance

		Non-cracked concrete						Cracked concrete					
Anchor size		**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M8**	**M10**	**M12**	**M16**	**M20**	**M24**
Tensile $N_{Ru,m}$	[kN]	31,1	39,2	47,9	66,9	93,5	122,9	15,9	21,2	34,2	47,8	66,8	87,8
Shear $V_{Ru,m}$													
HSL-3, HSL-3-B, HSL-3-SK a), HSL-3-SH a)	[kN]	*43,0*	*68,0*	95,8	133,8	187,0	*245,3*	40,0	56,0	68,4	95,6	133,6	175,6
HSL-3-G b)	[kN]	*36,1*	*48,1*	*75,1*	*118,5*	187,0	-	*36,1*	*48,1*	68,4	95,6	133,6	-

Characteristic resistance

		Non-cracked concrete						Cracked concrete					
Anchor size		**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M8**	**M10**	**M12**	**M16**	**M20**	**M24**
Tensile N_{Rk}	[kN]	23,4	29,5	36,1	50,4	70,4	92,6	12,0	16,0	25,8	36,0	50,3	66,1
Shear V_{Rk}													
HSL-3, HSL-3-B, HSL-3-SK a), HSL-3-SH a)	[kN]	*31,1*	*49,2*	*71,7*	100,8	140,9	*177,4*	30,1	42,2	51,5	72,0	100,6	132,3
HSL-3-G b)	[kN]	*26,1*	*34,8*	*54,3*	*85,7*	140,9	-	*26,1*	*34,8*	51,5	72,0	100,6	-

Design resistance

		Non-cracked concrete						Cracked concrete					
Anchor size		**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M8**	**M10**	**M12**	**M16**	**M20**	**M24**
Tensile N_{Rd}	[kN]	15,6	19,7	24,0	33,6	47,0	61,7	6,7	10,7	17,2	24,0	33,5	44,1
Shear V_{Rd}													
HSL-3, HSL-3-B, HSL-3-SK a), HSL-3-SH a)	[kN]	24,9	39,4	48,1	67,2	93,9	123,5	20,1	28,1	34,3	48,0	67,1	88,2
HSL-3-G b)	[kN]	20,9	27,8	43,4	67,2	93,9	-	20,1	27,8	34,3	48,0	67,1	-

Recommended loads

	Non-cracked concrete						Cracked concrete					
Anchor size	**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M8**	**M10**	**M12**	**M16**	**M20**	**M24**
Tensile N_{rec} [c] [kN]	11,2	14,1	17,2	24,0	33,5	44,1	4,8	7,6	12,3	17,1	24,0	31,5
Shear V_{rec} [c]												
HSL-3, HSL-3-B, HSL-3-SK[a], HSL-3-SH[a] [kN]	17,8	28,1	34,3	48,0	67,1	88,2	14,3	20,1	24,5	34,3	47,9	63,0
HSL-3-G[b] [kN]	14,9	19,9	31,0	48,0	67,1	-	14,3	19,9	24,5	34,3	47,9	-

a) HSL-3-SK and HSL-3-SH is only available up to M12
b) HSL-3-G is only available up to M20
c) With overall partial safety factor for action γ = 1,4. The partial safety factors for action depend on the type of loading and shall be taken from national regulations.

Materials

Mechanical properties of HSL-3, HSL-3-G, HSL-3-B, HSL-3-SH, HSL-3-SK

Anchor size		M8	M10	M12	M16	M20	M24
Nominal tensile strength f_{uk}	[N/mm²]	800	800	800	800	830	830
Yield strength f_{yk}	[N/mm²]	640	640	640	640	640	640
Stressed cross-section A_s	[mm²]	36,6	58,0	84,3	157	245	353
Moment of resistance W	[mm³]	31,3	62,5	109,4	277,1	540,6	935,4
Design bending resistance without sleeve $M_{Rd,s}$	[Nm]	24,0	48,0	84,0	212,8	415,2	718,4

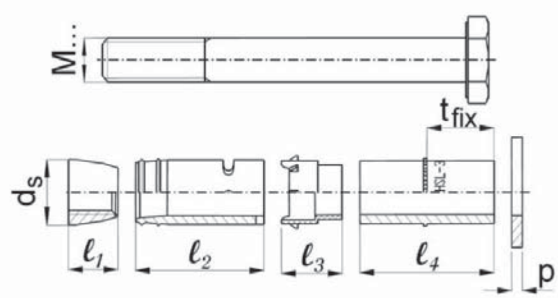
Material quality

Part	Material
Bolt, threaded rod	steel strength 8.8, galvanised to min. 5 µm

Anchor dimensions

Dimensions of HSL-3, HSL-3-G, HSL-3-B, HSL-3-SH, HSL-3-SK

Anchor version	Thread size	t_{fix} [mm]		d_s [mm]	ℓ_1 [mm]	ℓ_2 [mm]	ℓ_3 [mm]	ℓ_4 [mm]		p [mm]
		min	max					min	max	
HSL-3 HSL-3-G	M8	5	200	11,9	12	32	15,2	19	214	2
	M10	5	200	14,8	14	36	17,2	23	218	3
HSL-3 HSL-3-G HSL-3-B	M12	5	200	17,6	17	40	20	28	223	3
	M16	10	200	23,6	20	54,4	24,4	34,5	224,5	4
	M20	10	200	27,6	20	57	31,5	51	241	4
HSL-3 HSL-3-B	M24	10	200	31,6	22	65	39	57	247	4
HSL-3-SH	M8	5		11,9	12	32	15,2	19		2
	M10	20		14,8	14	36	17,2	38		3
	M12	25		17,6	17	40	20	48		3
HSL-3-SK	M8	10	20	11,9	12	32	15,2	18,2	28,2	2
	M10	20		14,8	14	36	17,2	32,2		3
	M12	25		17,6	17	40	20	40		3

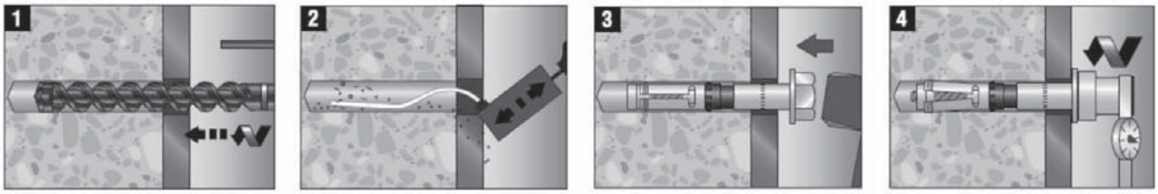


Setting

installation equipment

Anchor size	M8	M10	M12	M16	M20	M24
Rotary hammer	TE2 – TE16			TE40 – TE70		
Other tools	hammer, torque wrench, blow out pump					

Setting instruction

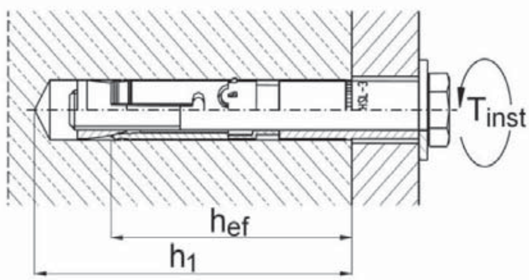


Drill hole. — Blow out dust and fragments. — Install anchor. — Apply tightening torque (for HSL-3-B: no torque wrench is needed)

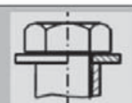
For detailed information on installation see instruction for use given with the package of the product.

For technical data for anchors in diamond drilled holes please contact the Hilti Technical advisory service.

Setting details: depth of drill hole h_1 and effective anchorage depth h_{ef}



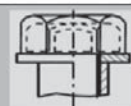
Setting details HSL-3

Anchor version HSL-3			M8	M10	M12	M16	M20	M24
Nominal diameter of drill bit	d_o	[mm]	12	15	18	24	28	32
Cutting diameter of drill bit	$d_{cut} \leq$	[mm]	12,5	15,5	18,5	24,55	28,55	32,7
Depth of drill hole	$h_1 \geq$	[mm]	80	90	105	125	155	180
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	14	17	20	26	31	35
Effective anchorage depth	h_{ef}	[mm]	60	70	80	100	125	150
Torque moment	T_{inst}	[Nm]	25	50	80	120	200	250
Width across	SW	[mm]	13	17	19	24	30	36

Setting details HSL-3-G

Anchor version HSL-3-G 			M8	M10	M12	M16	M20
Nominal diameter of drill bit	d_o	[mm]	12	15	18	24	28
Cutting diameter of drill bit	$d_{cut} \leq$	[mm]	12,5	15,5	18,5	24,55	28,55
Depth of drill hole	$h_1 \geq$	[mm]	80	90	105	125	155
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	14	17	20	26	31
Effective anchorage depth	h_{ef}	[mm]	60	70	80	100	125
Torque moment	T_{inst}	[Nm]	20	35	60	80	160
Width across	SW	[mm]	13	17	19	24	30

Setting details HSL-3-B

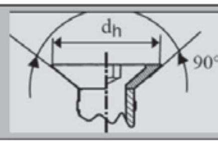
Anchor version HSL-3-B 			M12	M16	M20	M24
Nominal diameter of drill bit	d_o	[mm]	18	24	28	32
Cutting diameter of drill bit	$d_{cut} \leq$	[mm]	18,5	24,55	28,55	32,7
Depth of drill hole	$h_1 \geq$	[mm]	105	125	155	180
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	20	26	31	35
Effective anchorage depth	h_{ef}	[mm]	80	100	125	150
Width across	SW	[mm]	24	30	36	41

The torque moment is controlled by the safety cap

Setting details HSL-3-SH

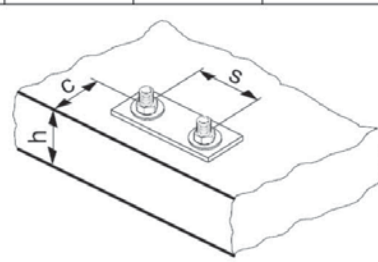
Anchor version HSL-3-SH 			M8	M10	M12
Nominal diameter of drill bit	d_o	[mm]	12	15	18
Cutting diameter of drill bit	$d_{cut} \leq$	[mm]	12,5	15,5	18,5
Depth of drill hole	$h_1 \geq$	[mm]	85	95	110
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	14	17	20
Effective anchorage depth	h_{ef}	[mm]	60	70	80
Torque moment	T_{inst}	[Nm]	25	35	60
Width across	SW	[mm]	6	8	10

Setting details HSL-3-SK

Anchor version HSL-3-SK			**M8**	**M10**	**M12**
Nominal diameter of drill bit	d_o	[mm]	12	15	18
Cutting diameter of drill bit	$d_{cut} \leq$	[mm]	12,5	15,5	18,5
Depth of drill hole	$h_1 \geq$	[mm]	80	90	105
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	14	17	20
Diameter of countersunk hole in the fixture	$d_h =$	[mm]	22,5	25,5	32,9
Effective anchorage depth	h_{ef}	[mm]	60	70	80
Torque moment	T_{inst}	[Nm]	25	50	80
Width across	SW	[mm]	5	6	8

Setting parameters

Anchor size			**M8**	**M10**	**M12**	**M16**	**M20**	**M24**
Minimum base material thickness	h_{min}	[mm]	120	140	160	200	250	300
Minimum spacing	s_{min}	[mm]	60	70	80	100	125	150
	for c ≥	[mm]	100	100	160	240	300	300
Minimum edge distance	c_{min}	[mm]	60	70	80	100	150	150
	for s ≥	[mm]	100	160	240	240	300	300
Critical spacing for splitting failure	$s_{cr,sp}$	[mm]	230	270	300	380	480	570
Critical edge distance for splitting failure	$c_{cr,sp}$	[mm]	115	135	150	190	240	285
Critical spacing for concrete cone failure	$s_{cr,N}$	[mm]	180	210	240	300	375	450
Critical edge distance for concrete cone failure	$c_{cr,N}$	[mm]	90	105	120	150	187,5	225



For spacing (edge distance) smaller than critical spacing (critical edge distance) the design loads have to be reduced.

Critical spacing and critical edge distance for splitting failure apply only for non-cracked concrete. For cracked concrete only the critical spacing and critical edge distance for concrete cone failure are decisive.

Simplified design method

Simplified version of the design method according ETAG 001, Annex C. Design resistance according data given in ETA-02/0042, issue 2013-01-10.

- Influence of concrete strength
- Influence of edge distance
- Influence of spacing
- Valid for a group of two anchors. (The method may also be applied for anchor groups with more than two anchors or more than one edge. The influencing factors must then be considered for each edge distance and spacing. The calculated design loads are then on the save side: They will be lower than the exact values according ETAG 001, Annex C. To avoid this, it is recommended to use the anchor design software PROFIS anchor)

The design method is based on the following simplification:

- No different loads are acting on individual anchors (no eccentricity)

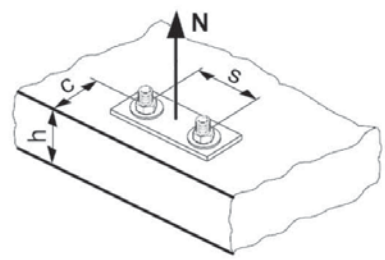
The values are valid for one anchor.

For more complex fastening applications please use the anchor design software PROFIS Anchor.

Tension loading

The design tensile resistance is the lower value of

- Steel resistance: $N_{Rd,s}$
- Concrete pull-out resistance: $N_{Rd,p} = N^0_{Rd,p} \cdot f_B$
- Concrete cone resistance: $N_{Rd,c} = N^0_{Rd,c} \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{re,N}$
- Concrete splitting resistance (only non-cracked concrete):
  $N_{Rd,sp} = N^0_{Rd,c} \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,sp} \cdot f_{re,N}$



Basic design tensile resistance

Design steel resistance $N_{Rd,s}$

Anchor size		M8	M10	M12	M16	M20	M24
$N_{Rd,s}$	[kN]	19,5	30,9	44,9	83,7	130,7	188,3

Design pull-out resistance $N_{Rd,p} = N^0_{Rd,p} \cdot f_B$ (only M8, M10 in cracked concrete)

		Non-cracked concrete						Cracked concrete					
Anchor size		M8	M10	M12	M16	M20	M24	M8	M10	M12	M16	M20	M24
$N^0_{Rd,p}$	[kN]	No pull-out failure						6,7	10,7	No pull-out failure			

Design concrete cone resistance $N_{Rd,c} = N^0_{Rd,c} \cdot f_B \cdot f_{1,N} \cdot f_{2,N} \cdot f_{3,N} \cdot f_{re,N}$
Design splitting resistance a) $N_{Rd,sp} = N^0_{Rd,c} \cdot f_B \cdot f_{1,sp} \cdot f_{2,sp} \cdot f_{3,sp} \cdot f_{h,sp} \cdot f_{re,N}$

		Non-cracked concrete						Cracked concrete					
Anchor size		M8	M10	M12	M16	M20	M24	M8	M10	M12	M16	M20	M24
$N^0_{Rd,c}$	[kN]	15,6	19,7	24,0	33,6	47,0	61,7	11,2	14,1	17,2	24,0	33,5	44,1

a) Splitting resistance must only be considered for non-cracked concrete

Influencing factors

Influence of concrete strength

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{1/2}$ a)	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of edge distance a)

$c/c_{cr,N}$ $c/c_{cr,sp}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$f_{1,N} = 0{,}7 + 0{,}3 \cdot c/c_{cr,N} \leq 1$ $f_{1,sp} = 0{,}7 + 0{,}3 \cdot c/c_{cr,sp} \leq 1$	0,73	0,76	0,79	0,82	0,85	0,88	0,91	0,94	0,97	1
$f_{2,N} = 0{,}5 \cdot (1 + c/c_{cr,N}) \leq 1$ $f_{2,sp} = 0{,}5 \cdot (1 + c/c_{cr,sp}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1

a) The edge distance shall not be smaller than the minimum edge distance c_{min} given in the table with the setting details. These influencing factors must be considered for every edge distance.

Influence of anchor spacing a)

$s/s_{cr,N}$ $s/s_{cr,sp}$	0,1	0,2	0,3	0,4	0,5	0,6	0,7	0,8	0,9	1
$f_{3,N} = 0{,}5 \cdot (1 + s/s_{cr,N}) \leq 1$ $f_{3,sp} = 0{,}5 \cdot (1 + s/s_{cr,sp}) \leq 1$	0,55	0,60	0,65	0,70	0,75	0,80	0,85	0,90	0,95	1

a) The anchor spacing shall not be smaller than the minimum anchor spacing s_{min} given in the table with the setting details. This influencing factor must be considered for every anchor spacing.

Influence of base material thickness

h/h_{ef}	2,0	2,2	2,4	2,6	2,8	3,0	3,2	3,4	3,6	≥ 3,68
$f_{h,sp} = [h/(2 \cdot h_{ef})]^{2/3}$	1	1,07	1,13	1,19	1,25	1,31	1,37	1,42	1,48	1,5

Influence of reinforcement

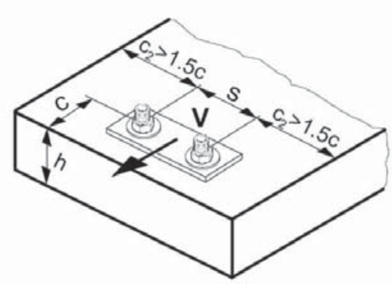
Anchor size	M8	M10	M12	M16	M20	M24
$f_{re,N} = 0{,}5 + h_{ef}/200mm \leq 1$	0,8 a)	0,85 a)	0,9 a)	1	1	1

a) This factor applies only for dense reinforcement. If in the area of anchorage there is reinforcement with a spacing ≥ 150 mm (any diameter) or with a diameter ≤ 10 mm and a spacing ≥ 100 mm, then a factor $f_{re,N}$ = 1 may be applied.

Shear loading

The design shear resistance is the lower value of

- Steel resistance: $V_{Rd,s}$
- Concrete pryout resistance: $V_{Rd,cp} = k \cdot N_{Rd,c}$
- Concrete edge resistance: $V_{Rd,c} = V^0_{Rd,c} \cdot f_B \cdot f_\beta \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$



Design steel resistance $V_{Rd,s}$

Anchor size		M8	M10	M12	M16	M20	M24
$V_{Rd,s}$	HSL-3, HSL-3-B, HSL-3-SK[a)], HSL-3-SH[a)] [kN]	24,9	39,4	57,4	80,9	113,5	141,9
	HSL-3-G [kN]	20,9	27,8	43,4	68,6	113,5	-

a) HSL-3-SK and HSL-3-SH is only available up to M12

Design concrete pryout resistance $V_{Rd,cp} = k \cdot N_{Rd,c}$ [a)]

Anchor size	M8	M10	M12	M16	M20	M24
k	1,8	2,0				

a) $N_{Rd,c}$: Design concrete cone resistance

Design concrete edge resistance [a)] $V_{Rd,c} = V^0_{Rd,c} \cdot f_B \cdot f_ß \cdot f_h \cdot f_4 \cdot f_{hef} \cdot f_c$

	Non-cracked concrete						Cracked concrete					
Anchor size	M8	M10	M12	M16	M20	M24	M8	M10	M12	M16	M20	M24
$V^0_{Rd,c}$ [kN]	11,7	16,9	22,9	36,8	47,7	59,7	8,3	12,0	16,2	26,1	33,8	42,3

a) For anchor groups only the anchors close to the edge must be considered.

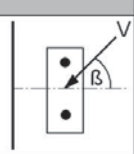
Influencing factors

Influence of concrete strength

Concrete strength designation (ENV 206)	C 20/25	C 25/30	C 30/37	C 35/45	C 40/50	C 45/55	C 50/60
$f_B = (f_{ck,cube}/25N/mm^2)^{1/2}$ [a)]	1	1,1	1,22	1,34	1,41	1,48	1,55

a) $f_{ck,cube}$ = concrete compressive strength, measured on cubes with 150 mm side length

Influence of angle between load applied and the direction perpendicular to the free edge

Angle ß	0°	10°	20°	30°	40°	50°	60°	70°	80°	≥ 90°
$f_\beta = \sqrt{\frac{1}{(\cos\alpha_V)^2 + \left(\frac{\sin\alpha_V}{2,5}\right)^2}}$ 	1	1,01	1,05	1,13	1,24	1,40	1,64	1,97	2,32	2,50

Influence of base material thickness

h/c	0,15	0,3	0,45	0,6	0,75	0,9	1,05	1,2	1,35	≥ 1,5
$f_h = \{h/(1,5 \cdot c)\}^{1/2} \leq 1$	0,32	0,45	0,55	0,63	0,71	0,77	0,84	0,89	0,95	1,00

Influence of anchor spacing and edge distance a) for concrete edge resistance: f_4

$f_4 = (c/h_{ef})^{1,5} \cdot (1 + s / [3 \cdot c]) \cdot 0,5$

c/h_{ef}	Single anchor	Group of two anchors s/h_{ef}														
		0,75	1,50	2,25	3,00	3,75	4,50	5,25	6,00	6,75	7,50	8,25	9,00	9,75	10,50	11,25
0,50	0,35	0,27	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35	0,35
0,75	0,65	0,43	0,54	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65	0,65
1,00	1,00	0,63	0,75	0,88	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00	1,00
1,25	1,40	0,84	0,98	1,12	1,26	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40	1,40
1,50	1,84	1,07	1,22	1,38	1,53	1,68	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84	1,84
1,75	2,32	1,32	1,49	1,65	1,82	1,98	2,15	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32	2,32
2,00	2,83	1,59	1,77	1,94	2,12	2,30	2,47	2,65	2,83	2,83	2,83	2,83	2,83	2,83	2,83	2,83
2,25	3,38	1,88	2,06	2,25	2,44	2,63	2,81	3,00	3,19	3,38	3,38	3,38	3,38	3,38	3,38	3,38
2,50	3,95	2,17	2,37	2,57	2,77	2,96	3,16	3,36	3,56	3,76	3,95	3,95	3,95	3,95	3,95	3,95
2,75	4,56	2,49	2,69	2,90	3,11	3,32	3,52	3,73	3,94	4,15	4,35	4,56	4,56	4,56	4,56	4,56
3,00	5,20	2,81	3,03	3,25	3,46	3,68	3,90	4,11	4,33	4,55	4,76	4,98	5,20	5,20	5,20	5,20
3,25	5,86	3,15	3,38	3,61	3,83	4,06	4,28	4,51	4,73	4,96	5,18	5,41	5,63	5,86	5,86	5,86
3,50	6,55	3,51	3,74	3,98	4,21	4,44	4,68	4,91	5,14	5,38	5,61	5,85	6,08	6,31	6,55	6,55
3,75	7,26	3,87	4,12	4,36	4,60	4,84	5,08	5,33	5,57	5,81	6,05	6,29	6,54	6,78	7,02	7,26
4,00	8,00	4,25	4,50	4,75	5,00	5,25	5,50	5,75	6,00	6,25	6,50	6,75	7,00	7,25	7,50	7,75
4,25	8,76	4,64	4,90	5,15	5,41	5,67	5,93	6,18	6,44	6,70	6,96	7,22	7,47	7,73	7,99	8,25
4,50	9,55	5,04	5,30	5,57	5,83	6,10	6,36	6,63	6,89	7,16	7,42	7,69	7,95	8,22	8,49	8,75
4,75	10,35	5,45	5,72	5,99	6,27	6,54	6,81	7,08	7,36	7,63	7,90	8,17	8,45	8,72	8,99	9,26
5,00	11,18	5,87	6,15	6,43	6,71	6,99	7,27	7,55	7,83	8,11	8,39	8,66	8,94	9,22	9,50	9,78
5,25	12,03	6,30	6,59	6,87	7,16	7,45	7,73	8,02	8,31	8,59	8,88	9,17	9,45	9,74	10,02	10,31
5,50	12,90	6,74	7,04	7,33	7,62	7,92	8,21	8,50	8,79	9,09	9,38	9,67	9,97	10,26	10,55	10,85

a) The anchor spacing and the edge distance shall not be smaller than the minimum anchor spacing s_{min} and the minimum edge distance c_{min}.

Influence of embedment depth

Anchor size	M8	M10	M12	M16	M20	M24
$f_{hef} = 0,05 \cdot (h_{ef} / d)^{1,68}$	0,75	0,67	0,61	0,55	0,62	0,67

Influence of edge distance a)

c/d	4	6	8	10	15	20	30	40
$f_c = (d / c)^{0,19}$	0,77	0,71	0,67	0,65	0,60	0,57	0,52	0,50

a) The edge distance shall not be smaller than the minimum edge distance c_{min}.

Combined tension and shear loading

For combined tension and shear loading see section “Anchor Design”.